EXISTING CONDITIONS REPORT

FOR

CARDINAL CREEK VILLAGE TAGGART GROUP OF COMPANIES

CITY OF OTTAWA

PROJECT NO.: 11-513

NOVEMBER 7, 2012 REVISION 2, 2ND SUBMISSION © DSEL

EXISTING CONDITIONS REPORT FOR CARDINAL CREEK VILLAGE

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EXISTING CONDITIONS REPORT FOR CARDINAL CREEK VILLAGE

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1.0 INTRODUCTION

This Existing Conditions Report is submitted in support of a City of Ottawa Official Plan Amendment for Cardinal Creek Village on behalf of the Taggart Group of Companies. The urban development site is a recent addition to the City of Ottawa urban area along the east edge of the urban area.

Cardinal Creek Village is approximately 225ha in size and is bound by Cardinal Creek to the west, existing residential development along Ted Kelly Lane to the east, Highway 174 and the Ottawa River to the north and Agricultural Lands to the south. Refer to *Figure 1* which depicts the study area and provides a key plan.

The proposed development is expected to provide municipal servicing to approximately 4800 residential dwellings, approximately 15,360 persons and an estimated 26.6ha of commercial / institutional lands.

This Existing Conditions Report is prepared to outline the existing water, sanitary, storm and stormwater management servicing for the development.

1.1 Existing Conditions

1.1.1 Environmental Features

The majority of Cardinal Creek Village is dominated by agricultural lands. Remnant forested parcels are to the south of Highway 174 and along the west and east edges of the site north of Old Montreal Road. The previously forested area south of Old Montreal Road has been removed with a remnant corridor remaining along an east-west tributary of Cardinal Creek and remnant hedgerows along the former forest edges.

The major natural environment features in the general area are the Cardinal Creek corridor to the west and the Ottawa River Corridor to the north.

Further information on the natural environment features within the study area of Cardinal Creek Village is provided in the Natural Environment Existing Conditions

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Report (Muncaster Environmental Planning, July 2012). The existing natural environment features are depicted on *Figure 2*.

1.1.2 Preliminary Geotechnical Investigation

Preliminary geotechnical investigations have been undertaken by Paterson Group, with all results of the investigation presented on *Figure 3*. A slope stability analysis has been completed to define the minimum setbacks from the existing drainage corridors.

The preliminary geotechnical investigation was based on a series of boreholes to provide testing on-site. The borehole locations are depicted on *Figure 3* as well as existing well locations.

The preliminary geotechnical investigation indicated that there is existing rock within the Cardinal Creek Village lands. The preliminary rock contours, provided by Paterson Group are depicted on *Figure 3*.

Full details are provided in the Supplemental Geotechnical Investigation prepared by Paterson Group on May 8, 2012.

2.0 GUIDELINES, PREVIOUS STUDIES, AND REPORTS

2.1 Existing Studies, Guidelines, and Reports

The following studies were utilized in the preparation of this report.

East Urban Community Sewer and Water Study McNeely Engineering Consultants Ltd, June 1992 (1992 McNeely Report)

East Urban Community Sewer Servicing Study Novatech Engineering Consultants Ltd, July 1997 (1997 Novatech Report)

Ottawa River Sub-Trunk and Trim Road Sanitary Sewer Assessment Stantec Consultants Ltd, February 28, 2001

City of Ottawa Official Plan Review, Urban Residential Land Needs, Municipal Servicing Review Taggart Cumberland Expansion Lands IBI Group, December 2008 (2008 IBI Report)

Engineering Servicing Evaluation Cardinal Village In Support of an Official Plan Amendment Submission David McManus Engineering Ltd, March 2009 (2009 DME Report)

Sewer Design Guidelines, City of Ottawa, November 2004 (City Standards)

Ottawa Design Guidelines – Water Distribution, City of Ottawa, July 2010 (Water Supply Guidelines)

City of Ottawa Official Plan, adopted by Council 2003. (Official Plan)

Stormwater Management Planning and Design Manual, Ministry of Environment, March 2003 (SWMP Design Manual)

Erosion & Sediment Control Guidelines for Urban Construction, Greater Golden Horseshoe Area Conservation Authorities, December 2006 (E&S Guidelines) 11-513

Technical Bulletin ISDTB-2012-1, City of Ottawa, January 31, 2012

Supplemental Geotechnical Investigation Proposed Cardinal Creek Residential Development Paterson Group, May 8, 2012 (*Paterson Report*)

Existing Conditions, Natural Environment Features, Cardinal Creek Village Muncaster Environmental Planning Inc, July 2012 (Muncaster Report)

3.0 EXISTING WATER SUPPLY SERVICES

Based on information contained in background reports, Cardinal Creek Village is located adjacent to the current boundary of the 1E and 2E Pressure Zones. As noted in the 2009 DME Report, given the site location and the adequate static pressure available, the lands should be connected to the 1E Pressure Zone infrastructure.

A summary of existing watermain locations in the vicinity of Cardinal Creek Village is presented in *Table 1*.

Table 1: Summary of Existing Watermains

Existing Watermain – Location	Size	Year Constructed
St. Joseph Boulevard, ending at Trim Road	406mm	1983
Trim Road from St. Joseph Boulevard to Taylor Creek Boulevard / Dairy Drive	406mm	1985
Dairy Road ROW from Trim Road to the cul- de-sac end of Dairy Road	406mm	1992
North Service Road from Trim Road to East Extent	406mm	1996
Watermain east of Trim Road from Dairy Road to the North Service Road	406mm	1996

The existing watermains are depicted on Figure 4.

Boundary conditions were requested from the City of Ottawa at three (3) locations as depicted on *Figure 4*:

- Location 1: St Joseph Boulevard and Trim Road;
- Location 2: Dairy Road, end of cul-de-sac (existing elevation 61.4m); and
- Location 3: North Service Road and East Extent (existing elevation 52.4m).

The City of Ottawa provided boundary condition information for Zone 1E, which does not include fire flow demand. The boundary conditions have been applied at each of the three locations to determine the range of pressures, which are presented in *Table 2* for existing conditions.

Existing (ass	uming Orleans	Watermain Lin	k is built)		
	Max HGL	Min HGL	Existing Grade	Max Pressure	Min Pressure
Location 1	115.1m	109.0m	65.6m	70.4psi (485.6kPa)	61.7psi (425.8kPa)
Location 2	115.1m	109.0m	61.4m	76.4psi (526.8kPa)	67.7psi (467.0kPa)
Location 3	115.1m	109.0m	54.2m	86.6psi (597.4kPa)	78.0psi (537.6kPa)

Table 2: Existing Watermain Pressures

Ministry of the Environment and City of Ottawa Guidelines indicate that it is best practice to have normal operating pressures between the range of 50psi to 80psi (345kPa to 552kPa). The information presented in Table 2 demonstrates that there are sufficient watermain pressures available at the connection points to Pressure Zone 1E for the Cardinal Creek Village to connect to.

Background studies have also concluded that Cardinal Creek Village can be adequately serviced by connecting to the existing municipal water distribution system. Further detailed hydraulic water servicing analysis will be completed as the project progresses.

4.0 WASTEWATER SERVICING

4.1 Existing Wastewater Services

The Cardinal Creek Village lands were included in the former City of Cumberland 50 year urban development expansion boundary, as noted in the 1992 McNeely Report. The 1992 McNeely Report also identified a 1200mm diameter trunk sewer named the "Cardinal East Trunk" and the "Ottawa River Sub Trunk", tying in at the intersection of

the existing Hydro corridor and Trim Road. The Cardinal Creek Village lands were included in the tributary sanitary drainage area to these trunk sewers.

Further to the 1992 McNeely Report, Novatech Engineering completed a Sewer Servicing Study in 1997, which recommended the "Ottawa River Sub Trunk" be sized as an 825mm diameter sewer (reduced from 1200mm). The 2001 Stantec Report was prepared to confirm and/or modify the recommendations in the 1997 Novatech Report due to changes in land uses and changes in the sanitary sewer system at that time. The 2001 Stantec Report recommended that the "Ottawa River Sub Trunk" sanitary pipe size form Tenth Line Road to Trim Road would need to be 900mm in size. The 900mm diameter trunk sewer what is constructed and is existing infrastructure.

The 2008 IBI Report identified a reserve sanitary capacity of approximately 195 L/s available in the Orleans Cumberland Collector (otherwise known as the "Ottawa River Sub Trunk").

A review of existing sanitary sewer infrastructure was undertaken to determine that the subject lands are well surrounded by existing sanitary sewers.

A summary of existing sanitary sewer locations in the vicinity of Cardinal Creek Village is presented in *Table 3*.

Table 3: Summary of Existing Sanitary Sewers

Existing Sanitary Sewer – Location	Size
Dairy Drive ROW from Trim Road to the cul-de-sac end of Dairy Drive	375mm
Trim Road ROW from Old Montreal Road to Dairy Drive	450mm / 525mm
Trim Road ROW from Dairy Drive across Highway 174 to the North Service Road	825mm

The existing sanitary sewers are depicted on *Figure 4*.

The City of Ottawa requested that the downstream sanitary trunk infrastructure be analyzed from Trim Road to Bilberry Creek, which varies in size from 900mm in diameter to 1350mm in diameter. The existing tributary area to this trunk sewer is

depicted on *Figure 5*. The as-built drawings for the trunk sewer from Trim Road to Bilberry Creek are enclosed in *Appendix A* for reference.

The City of Ottawa provided their GIS data for sanitary drainage areas. The GIS data was used to prepare a sanitary design sheet for the tributary area, which is enclosed in **Appendix B**.

Referring to the sanitary design sheet, the residual capacity in the trunk sewer is estimated as follows:

- ➤ 900mm diameter sewer at 0.12%, residual capacity = 345 L/s
- > 1200mm diameter sewer at 0.15%, residual capacity = 787 L/s
- > 1350mm diameter sewer at 0.08%, residual capacity = 487 L/s

4.2 Proposed Wastewater Demand

Preliminary sanitary sewer demand calculations have been completed for the Cardinal Creek Village lands. *Table 4* summarizes the City of Ottawa Design Guidelines which have been used to calculate the sanitary sewer demand calculations.

Design Parameter	Value
Low Density Residential	3.4p/unit
Medium Density Residential	2.7p/unit
Residential Average Flow	350L/p/d
Peaking Factor Applied	Harmon's Equation
Commercial / Institutional Flows	50,000 L/ha/day
Commercial / Institutional Peaking Factor	1.5
Industrial Flows	35,000 L/ha/day
Industrial Peaking Factor	Per Figure in Appendix 4-B
Infiltration and Inflow Allowance	0.28L/s/ha
Sanitary sewers are to be sized employing the Manning's Equation	$Q = \frac{1}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}}$
Extracted from Sections 4 and 6 of the City of Ottawa	Sewer Design Guidelines, November 2004.

Table 4: Wastewater Design Criteria

A sanitary design sheet has been prepared to estimate the sanitary flow from Cardinal Creek Village. The sanitary design sheet is enclosed in *Appendix B*.

The proposed flows from Cardinal Creek Village are made up of residential flows, commercial flows and institutional flows. The total residential peak flow is estimated at 172 L/s and the total commercial / institutional peak flow is estimated at 23.1 L/s. The total infiltration flow is estimated at 52.1 L/s. The total peak flow from Cardinal Creek Village is estimated at 247 L/s.

Based on the residual flow in the downstream trunk sewer, there is adequate sanitary capacity for Cardinal Creek Village.

5.0 STORM SERVICING

5.1 Existing Storm Drainage

Cardinal Creek Village lies primarily within the Cardinal Creek Subwatershed, with a small portion in the northeast located within Ottawa 1 Subwatershed.

The site is bisected by a tributary to Cardinal Creek, known as the "South Tributary" and abuts Cardinal Creek and the Ottawa River.

A Subwatershed Study is being completed by AECOM that will be available for public review in the fall of 2012.

The existing drainage catchments are depicted on *Figure 6*. The following information is provided to describe the methods used by JF Sabourin and Associates to delineate the catchments:

Using detailed Lidar point data and contour intervals, ArcGIS extension ArcHydro was used to generate the flow path network based on the existing conditions. Following the contours and flow paths, a manual delineation of the catchment areas and subwatersheds was created across the study area. There are 5 areas draining to the Cardinal Creek (areas 0,2,3,4 and 5) and 1 to the Ottawa River (area ID = 1). Field surveys and additional information may be acquired to augment the delineation and increase its precision.

5.2 Storm Servicing

Storm servicing to the site will be via stormwater management ponds on site which will outlet directly to Cardinal Creek or the Ottawa River. Pond outlet requirements for the ponds tributary to Cardinal Creek will be provided in the completed Subwatershed Study.

Generally, the ponds which discharge to the Ottawa River will be required to meet the following objectives:

- Not required to provide quantity control treatment.
- Quality control objectives will be to the MOE Enhanced level of protection.

Generally, the ponds which discharge to Cardinal Creek will be requird to meet the following objectives:

- Quantity control objectives pre = post
- Not to exceed erosion thresholds Quantity control objectives to the MOE Enhanced level of protections

6.0 CONCLUSION AND RECOMMENDATIONS

It has been determined that existing watermain and sanitary sewer infrastructure can support the Cardinal Creek Village development. Storm servicing will be completed by on-site stormwater management ponds discharging to both Cardinal Creek and the Ottawa River.

Further analysis will be completed as Concept Plans are developed.

Prepared by, Reviewed by,

David Schaeffer Engineering Ltd. David Schaeffer Engineering Ltd.

Per: Jennifer Ailey, P.Eng. Per: Stephen Pichette, P.Eng.

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APPENDIX A As-Built Drawings

		SYMBO	VI C		DEEEDENOE DOINTS
				D5140141.0	REFERENCE POINTS
		EXISTING_	PROPOSED	REMOVALS	⊕BM BENCH MARK
				CONC ASPH	CONCRETE MONUMENT
	Edge of Road & Type 1:500				■ ROCK BAR
	Concrete Curb Type 1 : 1000			-*-*-* -	O IP IRON TUBE OR PIPE
	Concrete Curb Type 1 : 500		ROAD OR PARKING SIDE		D 18 IRON BAR
	Ditches and Creeks etc. over 0.6m wide				II WS WOOD STAKE
	Ditch				ISCM 2- 2ND ORDER INTEGRATED SURVEY CONTROL MONUMENT
	Storm Sewer & Monhole	MH			ISCM 3- 3RD ORDER INTEGRATED SURVEY CONTROL MONUMENT CUT CROSS
	Son. Sewer & Monhole Water Main & Volve Chamber		_vc		V CUT VEE
	Goe Moin & Volve				O RIB ROUND IRON BAR
	Rogers Cable Conduit & Monhole	MH I —			■ REINFORCING BAR
	Rogers Fibre Optic Conduit & Monhole	MH F			· D SIB STANDARD IRON BAR
	Culvert				III SSIB SHORT STANDARD IRON BAR
	Culvert with Headwolle		D	 * - = =	⊕ WORK POINT
	Bell Telephone Conduit & Monhole			,	RAILWAYS
	Hydro Conduit & Manhole	MH — H — — —	_о <mark>мн</mark> н	!	IMILWATS
	Hydro Street Lighting Conduit			×	1:500, 1:1000 SCALE, SINGLE (MULTIPLE) TRACK
	Single Catch Basin or Ditch Inlet	ט	•	©	
	Double Catch Basin	DC	==	. ♦ ⊕	1
	Hydrant & Valve Box	~8 ↓	~ <u>₩</u>	- -⊗	WW WIG - WAG
	Bell Utility Pole & Anchor	<○ 8	(BY OTHERS) C——O B	⊸	A WWB WIG - WAG AND BELL
	Hydro Utility Pole & Anchor	со н	(BY 07HERS) C	← ⊚	X RCS RAILWAY CROSSING SIGN
	Street Light	o-	~		. To FLASHING LIGHT
	Traffic Manhole	0	۰		1
4	Traffic Handhole		-	⊖``````	*
	"170" Traffic Controller Foundation	8	•	®	
	Traffic Most Arm Foundation	•	*	⊕ '	MISCELLANEOUS
	Traffic Tubular Foundation	0	•	0	
	Joint Use Pole Foundation	®	•	©	AREA TO BE CLEARED
,	Traffic Controller Foundation	8	•	•	······
	Detector Loop	0			AREA TO BE GRUBBED
	51 mm (2") Conduit, Conc. Encased	<i></i>		*X- * * X -	AREA TO BE CLEARED AND GRUBBED
	76mm (3") Conduit, Conc. Encased			**	
	102mm (4") Conduit, Conc. Encased	, , , ,		** - *	EDGE OF LAKE OR RIVER
	127mm (5") Conduit, Conc. Encomed	F_71	<u> </u>		SWAMP AND EDGE OF SWAMP
	Steel Hydro Tower Trees		\odot	. ⊗ ⊠	TATATAL ROCK SECTION IN PROFILE
	Hedge			6222222	(1) IA
	Sush Area		execute.		ROADWAY GRADING
	Property Line			4777777	
	Centre Line				- INT - FILL
	Reference Point H. O. T.				OSF OVERHEAD SIGN FOOTING
	P. I. (Point of Intersection)				OS OVERHEAD SIGNS
	Fence & Gate	-x	(MFE)	-x-0 X 3 (TYPE)	
	Guide Rall		(MPE)	-x	-Q- BOREHOLE
	Retaining Wall	RET WALL	(TIPE)	X RET. WALL X	r
	Adjust Surfoce from Works	O MH	9 ¹⁰⁴ 13		FOUNDATION ONLY
	Sidewolide ASPN.	==== =	STATE OF THE PARTY OF THE PARTY.	enter (
	Roadway, Lanewaye & Entrances ASPIL		COMC. ASPH.	(ASPH)	BUILDINGS
	Perforated Pipe Sub - Drain	,	(MZE)		
	Concrete Precost Curb				1





Cumming Cockburn Limited



NO.	REVISIONS	BY
0	ISSUED FOR MOEE APPROVAL	R.W.W.
1	ISSUED FOR CONSTRUCTION	R.W.W.

VISIONS	BY	DATE
MOEE APPROVAL	R.W.W.	01:00:31
R CONSTRUCTION	R.W.W.	01: 11: 19
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OTTAWA RIVER SUB TRUNK
NORTH SERVICE ROAD CUMBERLAND WARD

CONTRACT NO. ETL01-2113 DWG. NO. 10940-1

LEGEND AND INDEX W. NEWELL, P.Eng.

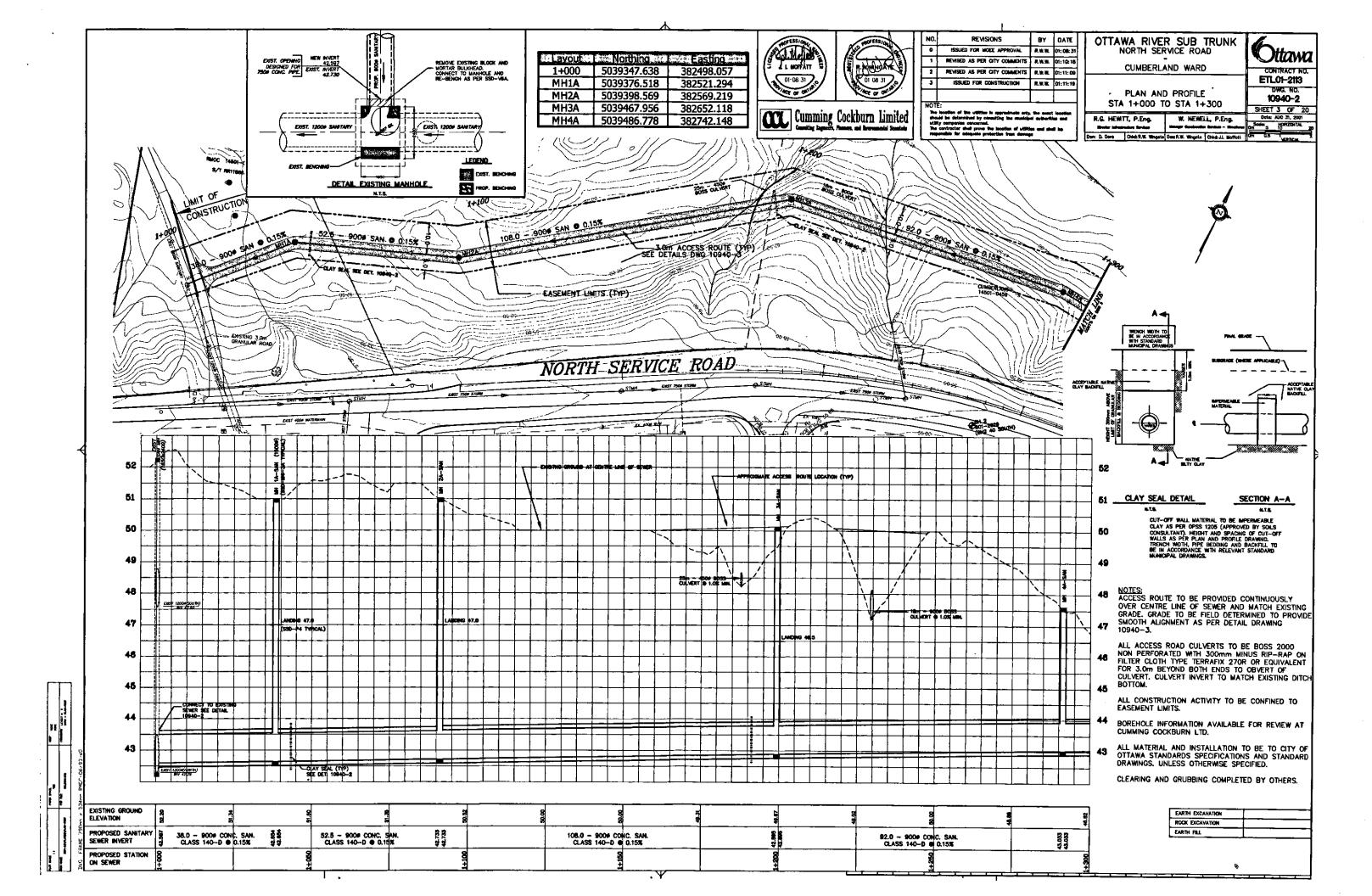
SHEET 2 OF 20 Date: AUG 31, 2001 Bode: M.Y.S.

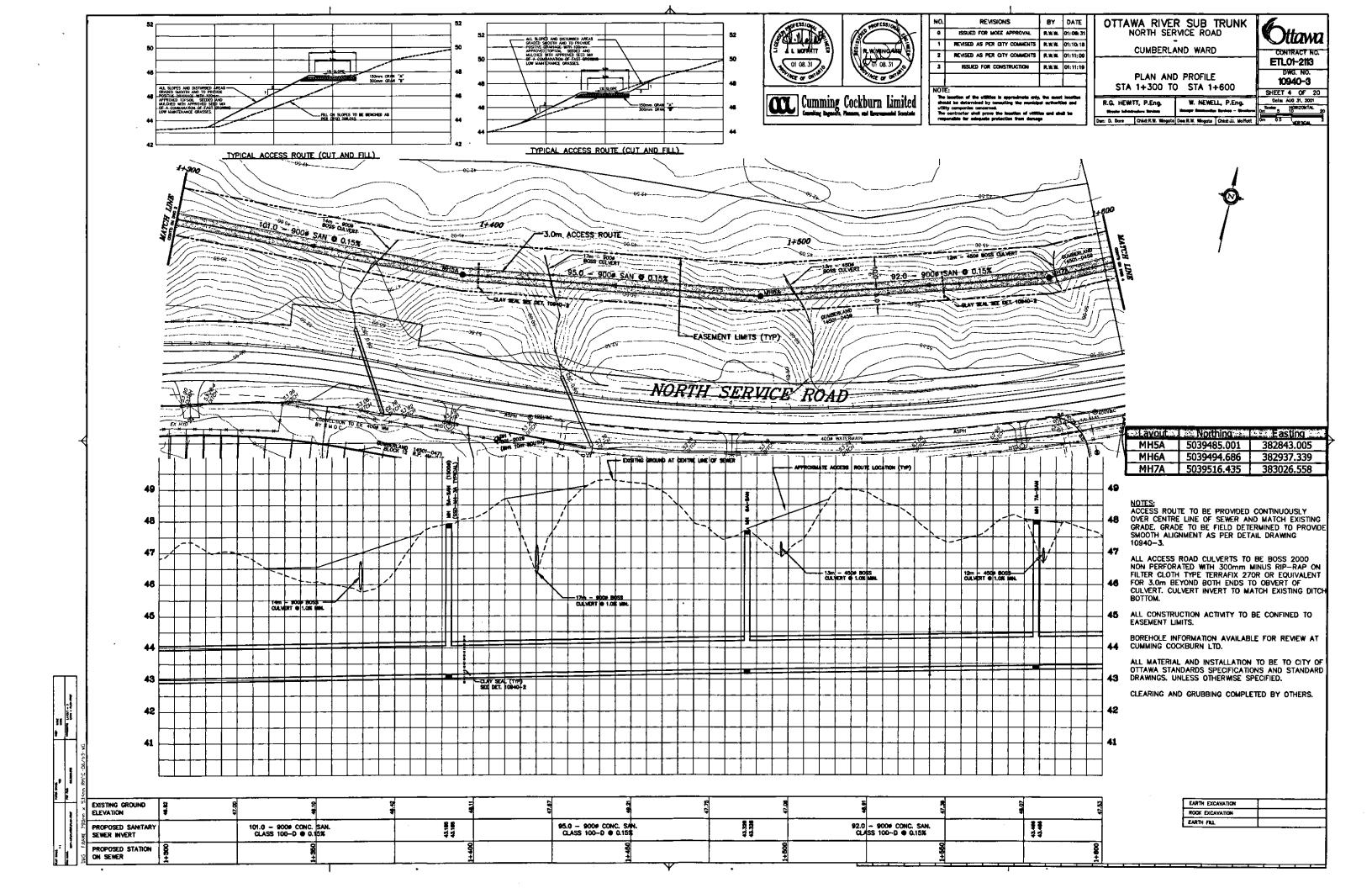
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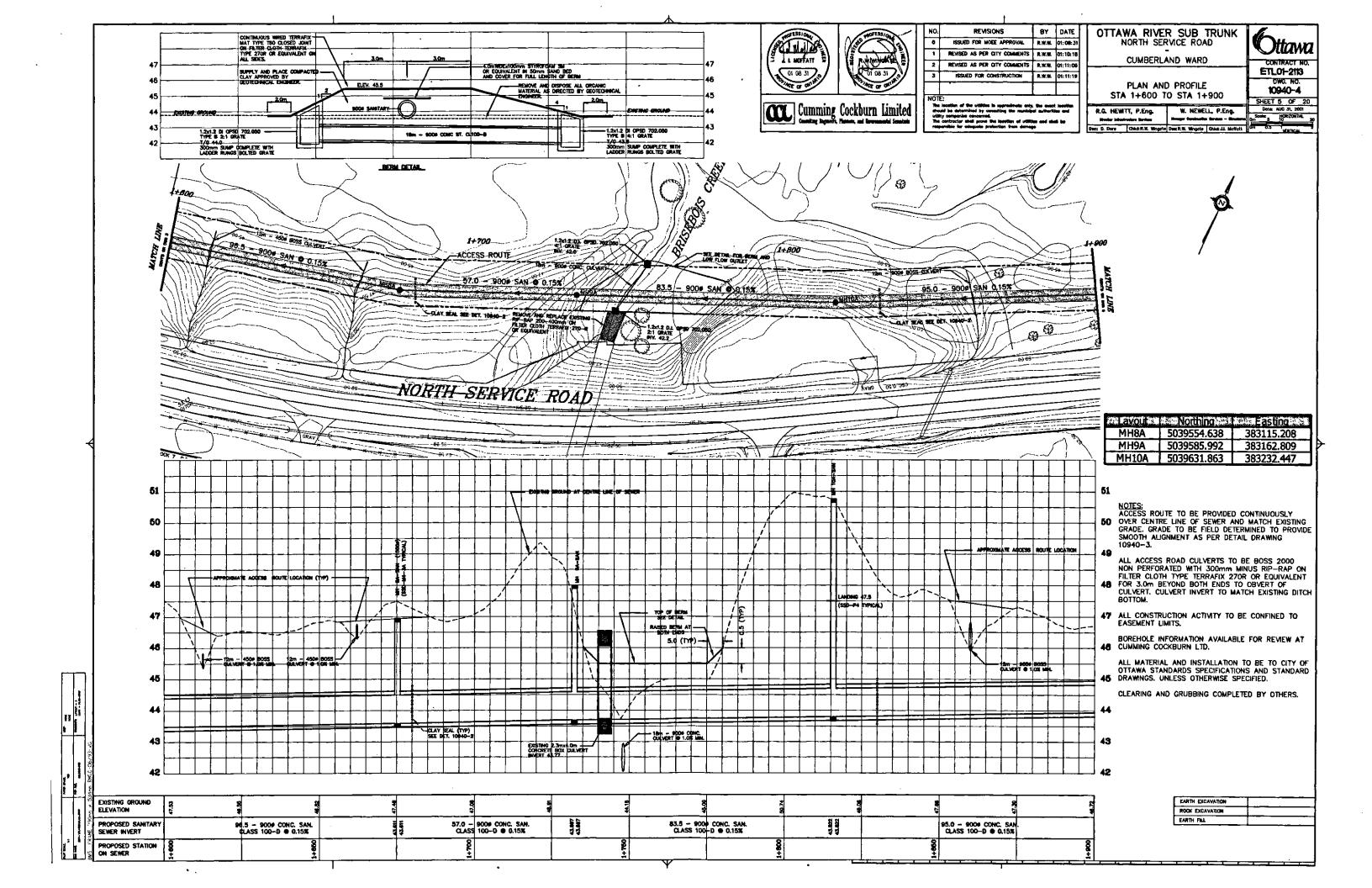
DRAWING NO.	DESCRIPTION
	<u> </u>
10940-0	COVER SHEET
10940-1	LEGEND AND INDEX
10940~2	STA, 1+000 TO STA, 1+300 NORTH SERVICE ROAD
109403	STA. 1+300 TO STA. 1+600 NORTH SERVICE ROAD
109404	STA. 1+600 TO STA. 1+900 NORTH SERVICE ROAD
10940-5	STA, 1+900 TO STA, 2+200 NORTH SERVICE ROAD
10940-6	STA. 2+200 TO STA. 2+500 NORTH SERVICE ROAD
10940-7	STA. 2+500 TO STA. 2+800 NORTH SERVICE ROAD
10940-8	STA. 2+800 TO STA. 3+100
109409	STA. 3+100 TO STA. 3+400
1094010	STA. 3+400 TO STA. 3+800 TRIM ROAD
1094011	STA. 0+000 TO STA. 0+300 NORTH SERVICE ROAD
1094012	STA. 0+900 TO STA. 0+175 PUMP STATION TO MH 29A
10940-13	EROSION AND SEDIMENT CONTROL PLAN
10940-14	EROSION AND SEDIMENT CONTROL PLAN
10940-15	HEADWALL STRUCTURE STA 0+066 GENERAL ARRANGEMENT
1094016	HEADWALL STRUCTURE STA 0+066 REINFORCING DETAILS
10940-17	HEADWALL STRUCTURE STA 2+008.5 GENERAL ARRANGEMENT
10940-18	HEADWALL STRUCTURE STA 2+008.5 REINFORCING DETAILS
10940-19	Tunnel Boring and Sewer Pipe Installation Sta 3+078.25
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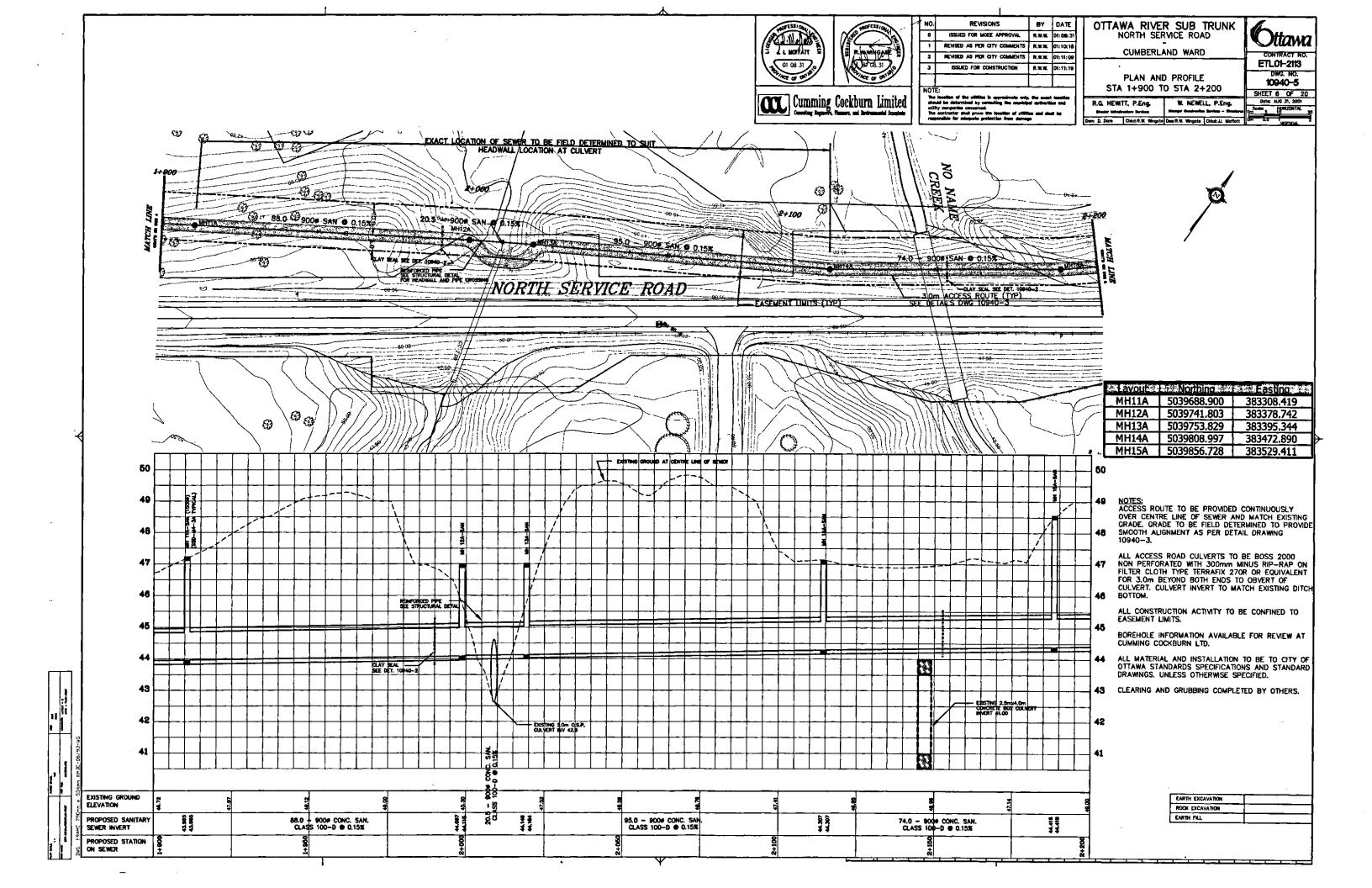


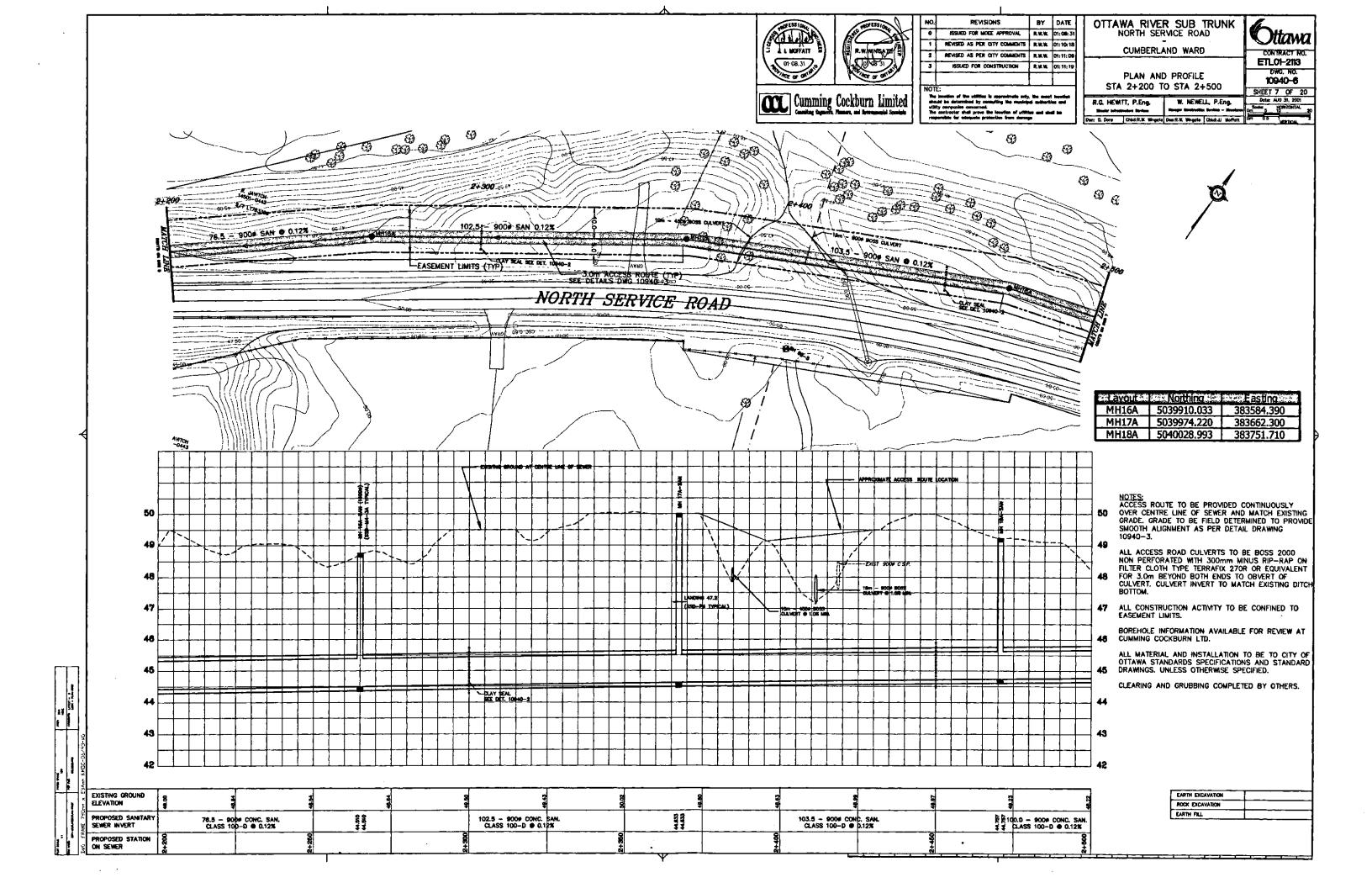
3.0m granular access route see DWG __10940-3 FOR DETAILS

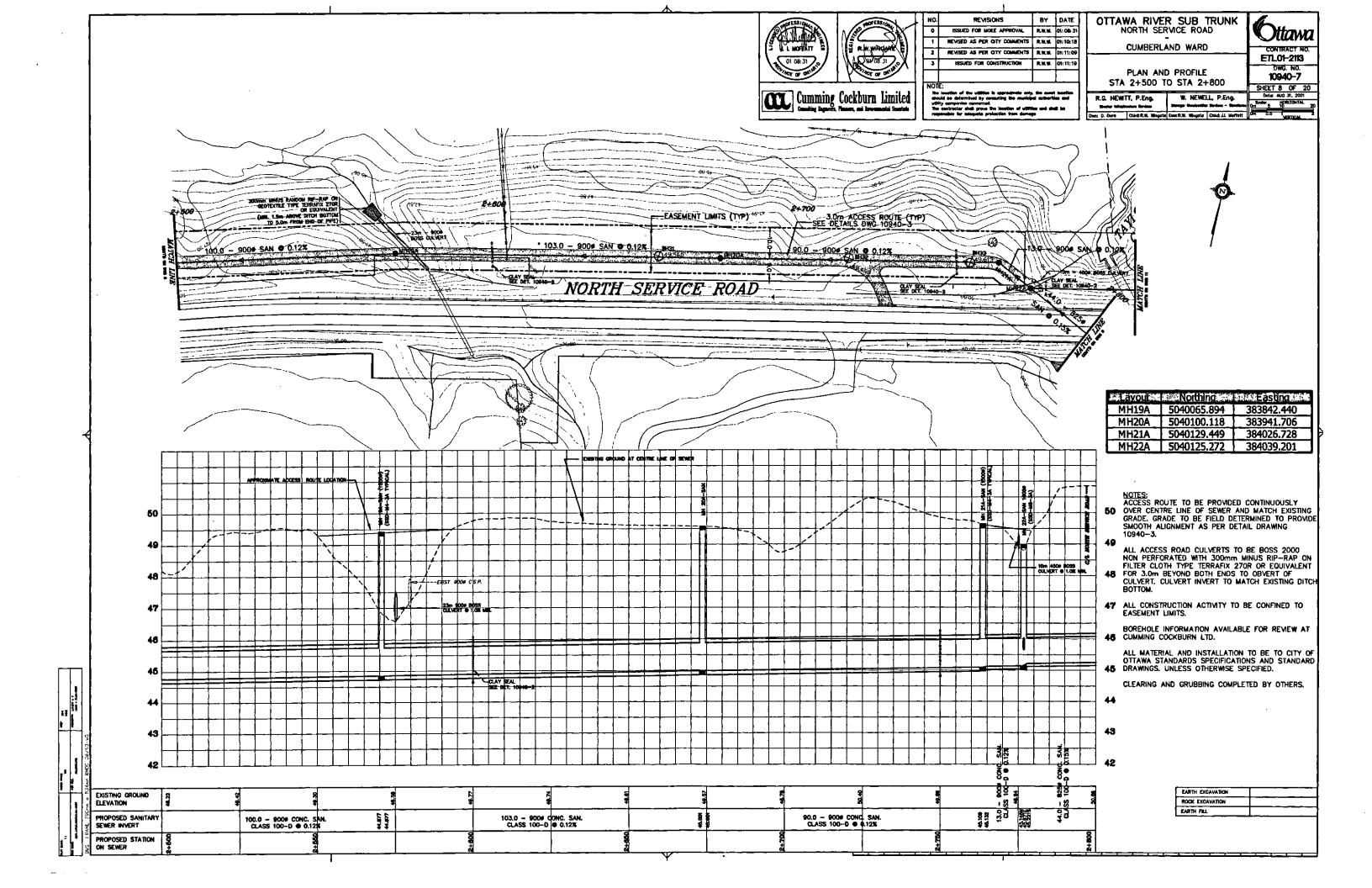


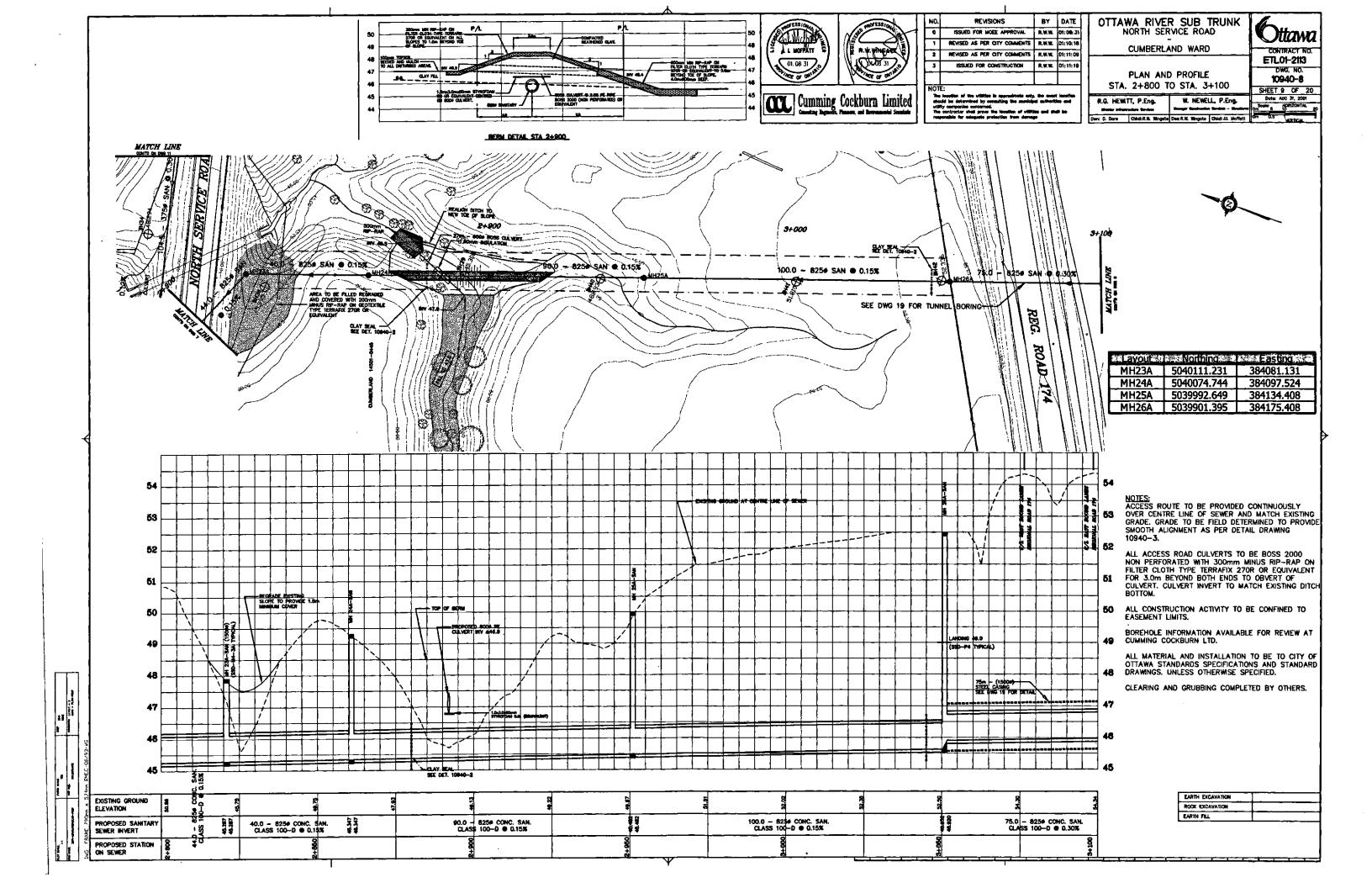


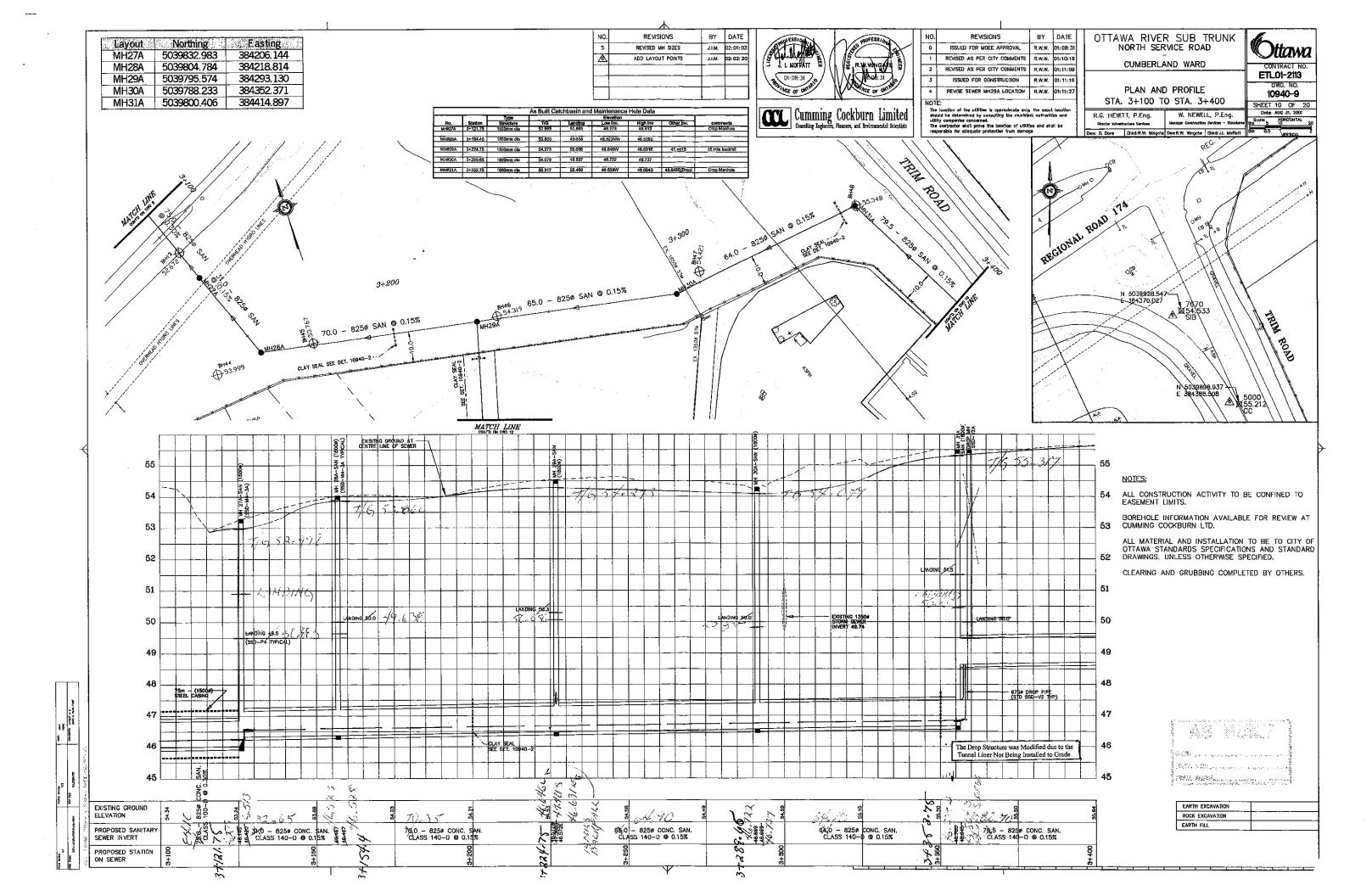


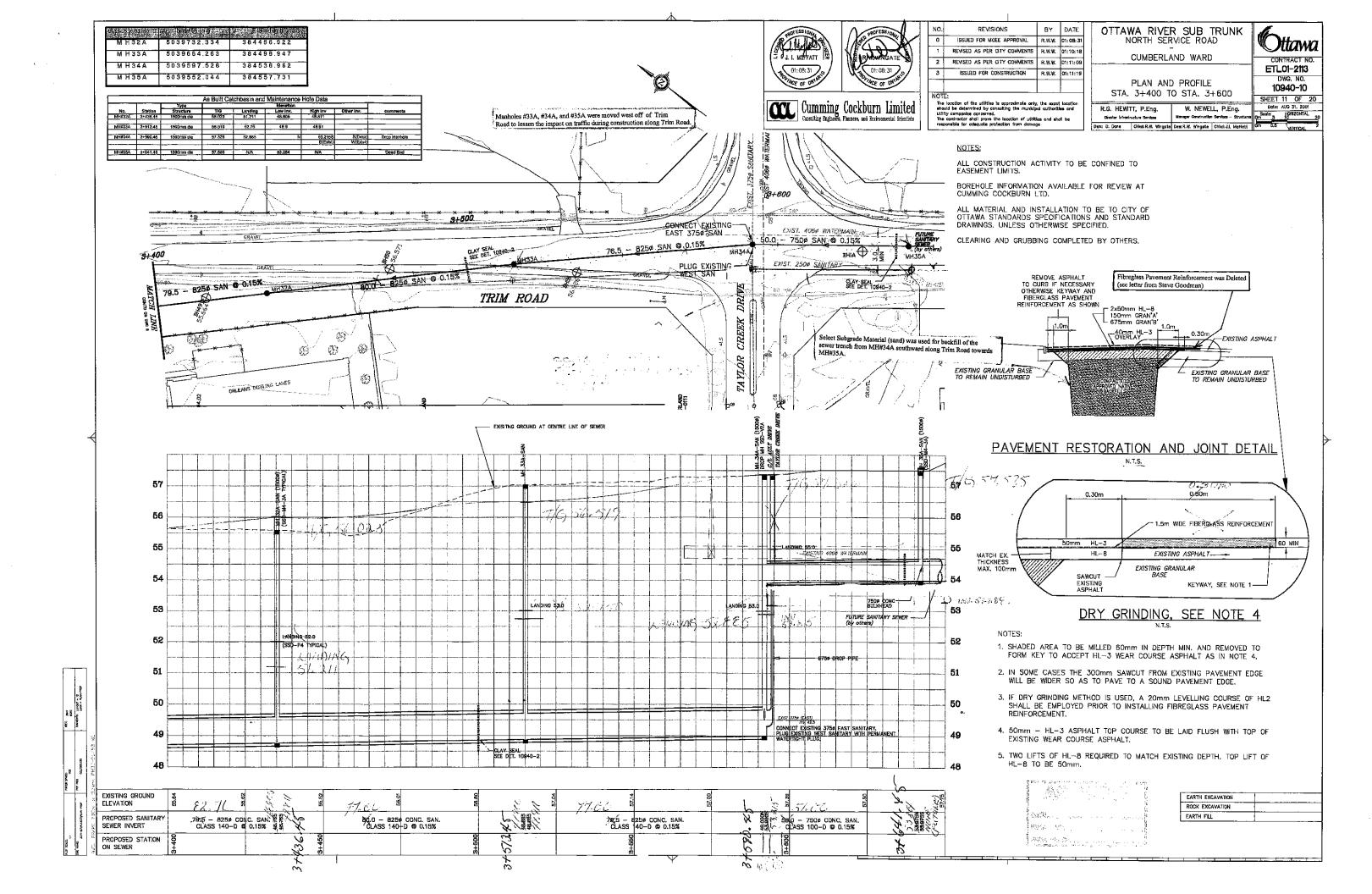


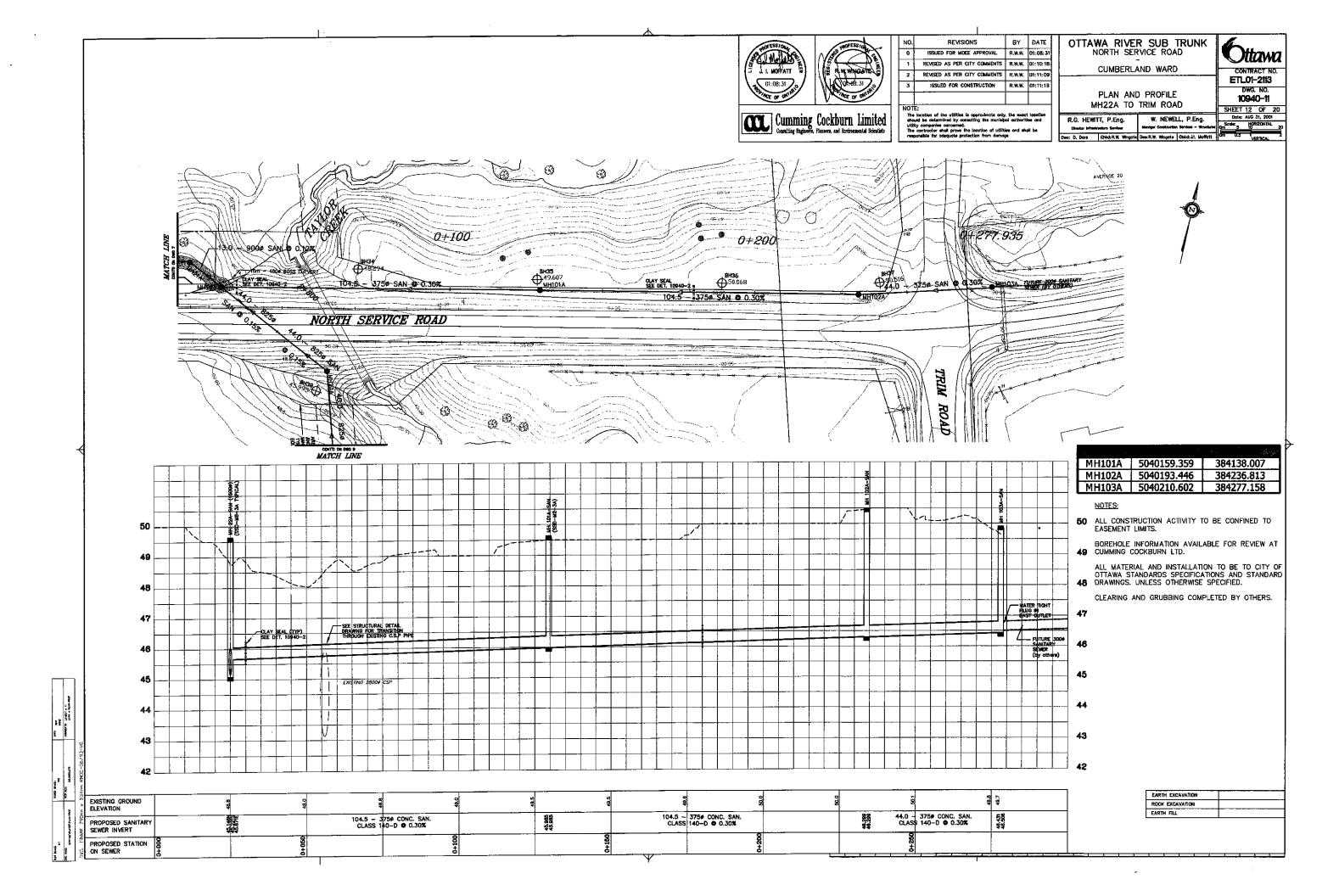


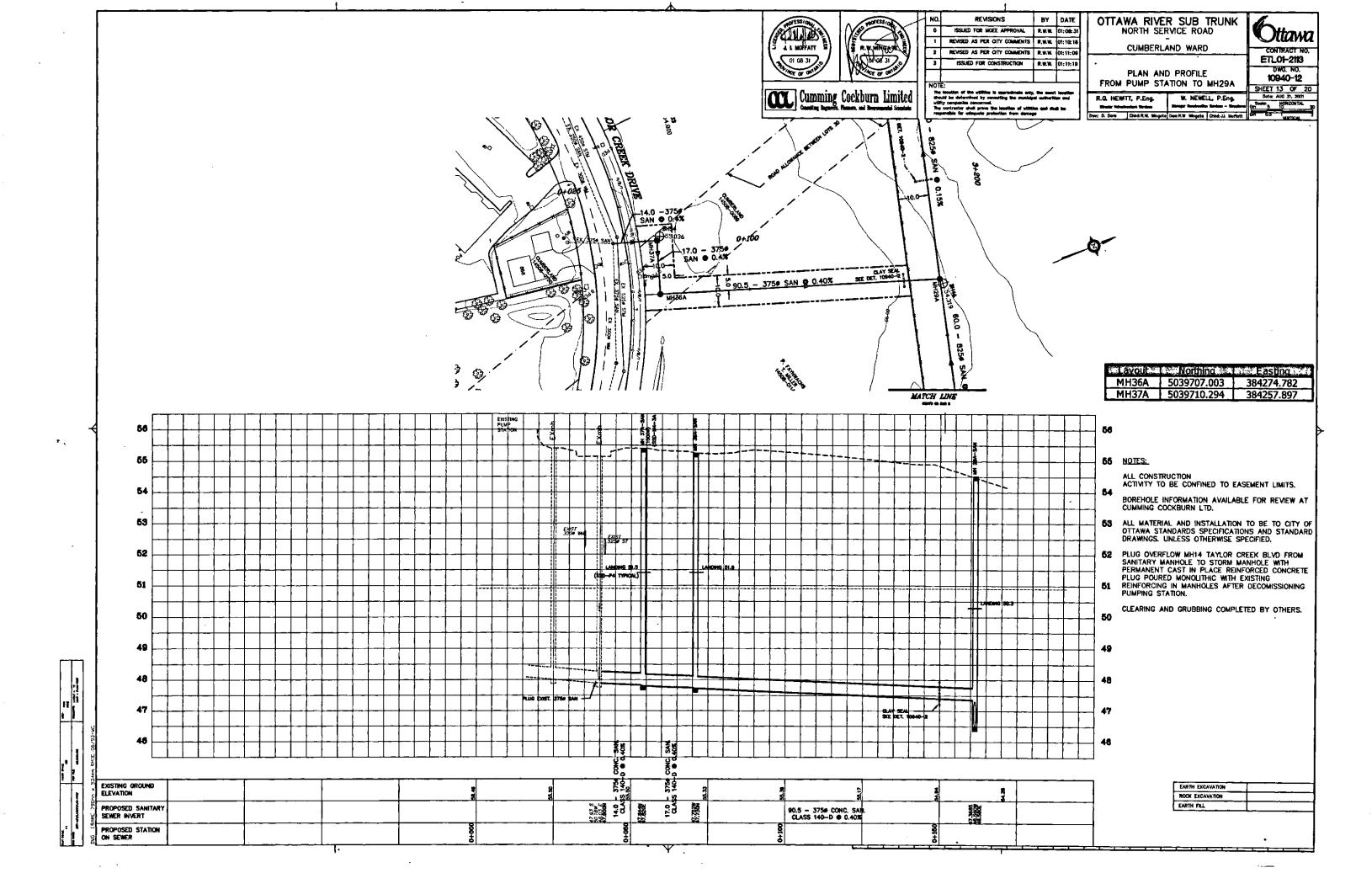


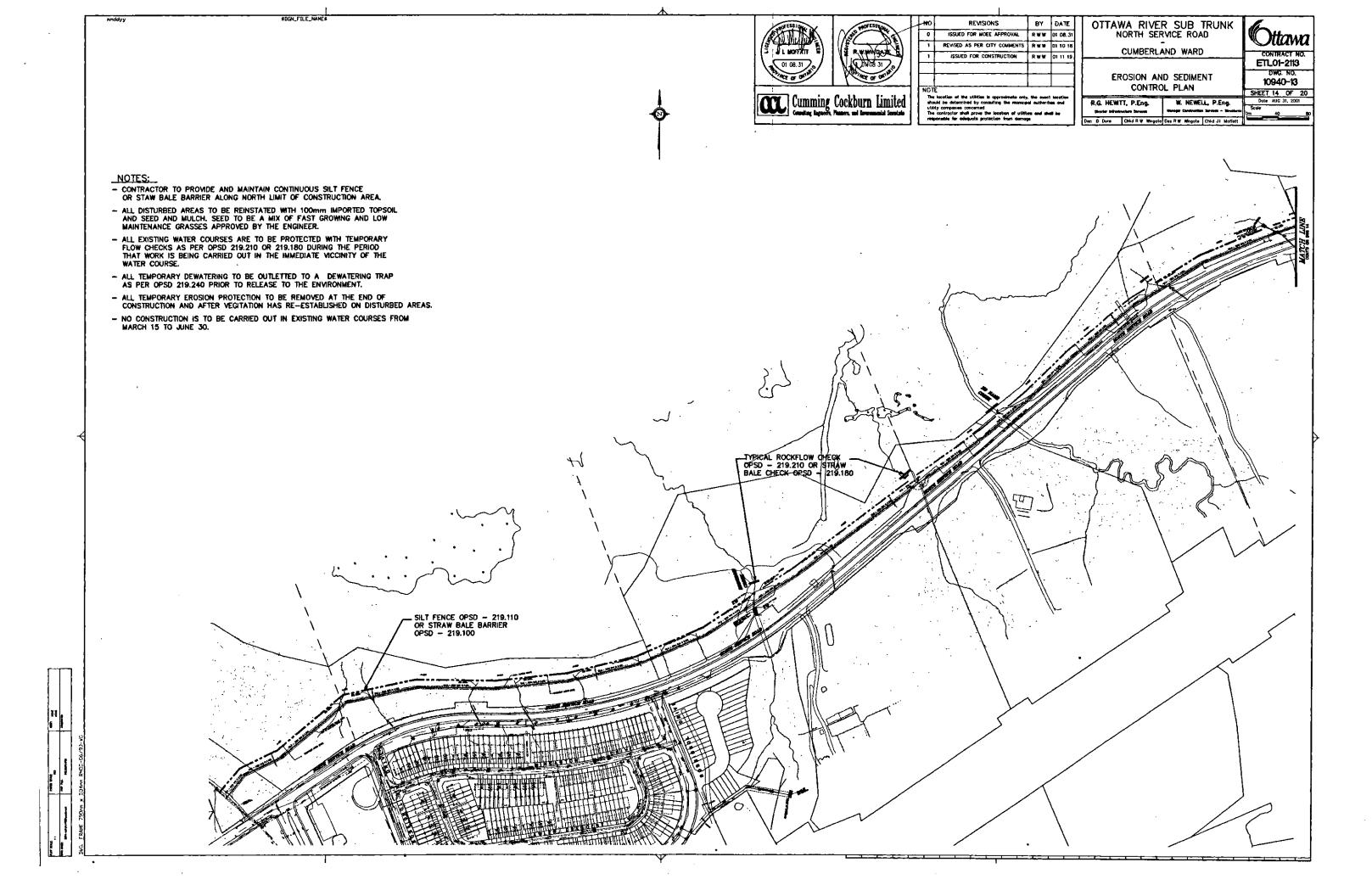


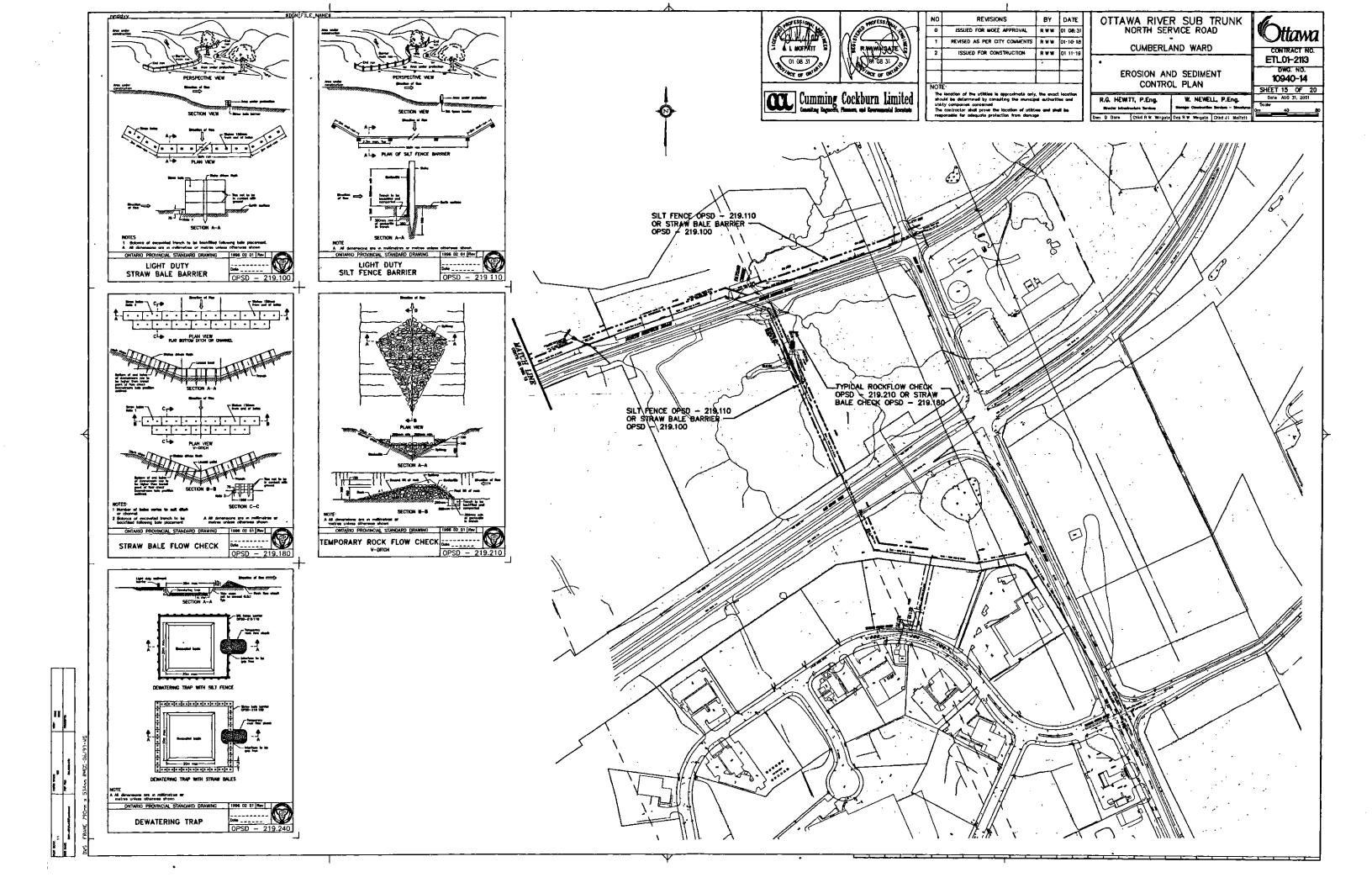












APPENDIX B Sanitary Design Sheets

SANITARY SEWER CALCULATION SHEET - EXISTING FLOW

Cardinal Creek Village PROJECT: 11-513

20-Jul-12

LOCATION: FILE REF:

DATE:

DESIGN PARAMETERS

Avg. Daily Flow Instit.

Avg. Daily Flow Res. 350 L/p/d

Avg. Daily Flow Comm. 50,000 L/ha/d

50,000 L/ha/d

Peak Fact. Comm. 1.5 1.5 Peak Fact. Instit.

Infiltration / Inflow 0.28 L/s/ha 0.60 m/s full flowing Min. Pipe Velocity Max. Pipe Velocity 3.00 m/s full flowing

Avg. Daily Flow Indust. 35,000 L/ha/d Peak Fact. Indust. per MOE graph Mannings N

Peak Fact Res. Per Harmons: Min = 2.0, Max =4.0



	Location			Resider	ntial Area	a and Popi	ulation		Comm	ercial	Institu	ıtional	Indu	strial			Infiltration						Pipe	Data			
Area ID	Up	Down	Area	Pop.	Cum	ulative	Peak.	Q _{res}	Area	Accu.	Area	Accu.	Area	Accu.	Q _{C+I+I}	Total	Accu.	Infiltration	Total	DIA	Slope	Length	A _{hvdraulic}	R	Velocity	Q _{cap}	Q / Q full
					Area	Pop.	Fact.			Area		Area		Area		Area	Area	Flow	Flow				,				
			(ha)		(ha)		(-)	(L/s)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(L/s)	(ha)	(ha)	(L/s)	(L/s)	(mm)	(%)	(m)	(m²)	(m)	(m/s)	(L/s)	(-)
V			7.500	575.0		·	3.94	9.18		0.00		0.00		0.00	0.0	7.500	7.500	2.100	11.28								
W			61.400			·	3.26	64.11	1.60	1.60			1.10		15.8	79.700	87.200	24.416	104.35								
X			85.100			12515.0	2.86	144.86	4.80	6.40	13.80			1.10	32.0	103.700	190.900	53.452	230.28								
Υ			30.900	2156.0	184.900	14671.0	2.79	165.69		6.40		29.40		1.10	32.0	30.900	221.800	62.104	259.76								
L	1	2	11.000	118.0	195.900	14789.0	2.78	166.81	1.10	7.50		29.40	15.80	16.90	45.7	27.900	249.700	69.916	282.45	900	0.12		0.636	0.225	0.99	627.1	0.45
1			12.000	608.0	12 000	15397.0	2.77	172.57		7.50		29.40		16.90	45.7	12.000	261.700	73.276	291.57			 	-		 		
K			0.000			15397.0	2.77	172.57	8.80			29.40		16.90	53.4	8.800	270.500	75.740	301.67				-		 		
M			1.100			15411.0	2.77	172.70	4.30		6.00			16.90	62.3	11.400	281.900	78.932	313.94			 	-		 		
N		-		14921.0			2.47	303.81	11.30	31.90	0.00	35.40	15.50	32.40	84.7	255.800	537.700	150.556	539.03			ł	1				+
R			34.900			32444.0	2.44	321.20	15.80	47.70		35.40	5.70	38.10	103.0	56.400	594.100	166.348	590.55			-					+
S			1.800			32465.0	2.44	321.37	10.00	47.70		35.40	0.70	38.10	103.0	1.800	595.900	166.852	591.22								+
T			44.000			36381.0	2.40	353.05	10.90	58.60		35.40	8.80	46.90	119.6	63.700	659.600	184.688	657.34			-					
U	2	3	44.000			40297.0	2.35	384.09	10.90	69.50		35.40	8.80	55.70	136.2	63.700	723.300	202.524	722.80	1200	0.15		1.131	0.300	1.34	1510.0	0.48
D			4.700	238.0	4.700	40535.0	2.35	385.96		69.50		35.40		55.70	136.2	4.700	728.000	203.840	725.98								
E			14.200	656.0	18.900	41191.0	2.34	391.09		69.50		35.40		55.70	136.2	14.200	742.200	207.816	735.10								
F			6.000	453.0	24.900	41644.0	2.34	394.63		69.50		35.40		55.70	136.2	6.000	748.200	209.496	740.31								
G			12.900	571.0	37.800	42215.0	2.33	399.08		69.50		35.40		55.70	136.2	12.900	761.100	213.108	748.38								
Н			9.900	793.0	47.700	43008.0	2.33	405.24		69.50		35.40		55.70	136.2	9.900	771.000	215.880	757.31								
I			8.400	663.0	56.100	43671.0	2.32	410.37		69.50		35.40		55.70	136.2	8.400	779.400	218.232	764.79								
0			71.700	4705.0	127.800	48376.0	2.28	446.40	3.20	72.70	3.30	38.70		55.70	141.8	78.200	857.600	240.128	828.36								
P			86.300	5614.0	214.100	53990.0	2.23	488.54	20.70	93.40	8.10	46.80		55.70	166.8	115.100	972.700	272.356	927.72								
Q	3	4	34.900	2112.0	249.000	56102.0	2.22	504.17	15.80	109.20		46.80	5.70	61.40	185.2	56.400	1029.100	288.148	977.48	1350	0.08		1.431	0.338	1.05	1509.6	0.65
Λ		-	21.800	1607.0	270 900	57709.0	2.21	516.00	0.40	109.60		46.80	2.20	63.60	187.3	24.400	1053.500	294.980	998.27			 			-		
Α						·			0.40																 		
<u>D</u>		OUTLET	27.600			59295.0	2.20	527.61		109.60		46.80	2.20		189.1		1083.300		1020.01	1250	0.00		1 424	0.220	1.05	1500.0	
U	4	OUTLET	3.800	190.0	302.200	59485.0	2.20	529.00		109.60		46.80		65.80	189.1	3.800	1087.100	304.388	1022.46	1350	0.08		1.431	0.338	1.05	1509.6	0.68

SANITARY SEWER CALCULATION SHEET - PROPOSED DEVELOPMENT

PROJECT: Cardinal Creek Village DESIGN PARAMETERS

LOCATION: Avg. Daily Flow Res. 350 L/p/d Peak Fact Res. Per Harmons: Min = 2.0, Max = 4.0 Infiltration / Inflow 0.28 L/s/ha

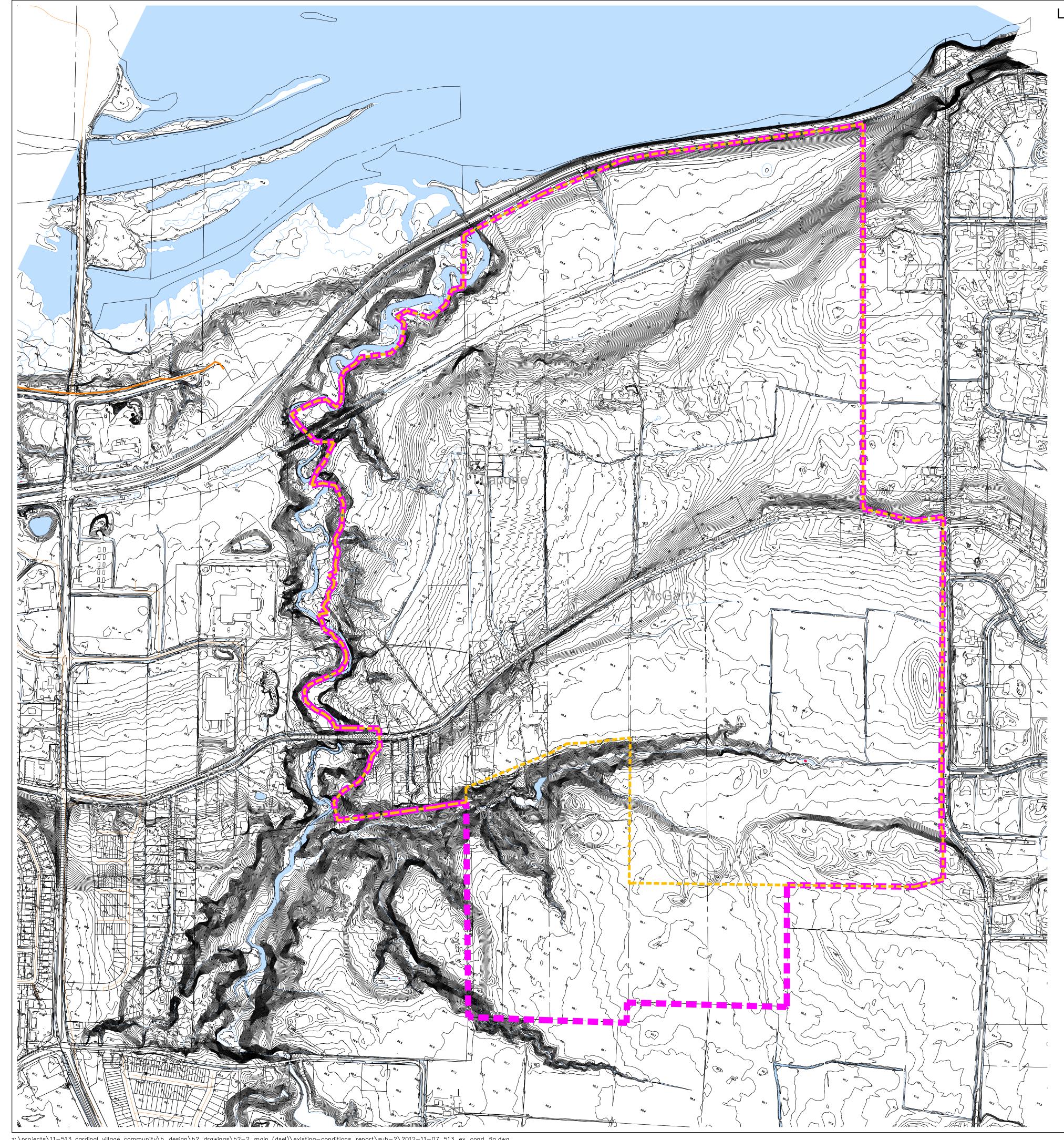
FILE REF: 12-513 1.5 0.60 m/s full flowing Avg. Daily Flow Com 50,000 L/ha/d Peak Fact. Comm. Min. Pipe Velocity 20-Jul-12 1.5 DATE: Avg. Daily Flow Instit 50,000 L/ha/d Peak Fact. Instit. Max. Pipe Velocity 3.00 m/s full flowing

Avg. Daily Flow Indu: 35,000 L/ha/d Peak Fact. Indust. per MOE graph Mannings N 0.013



	Residenti	al Area a	and Popu	lation		Comm	nercial	Institu	tional	Indus	strial			Infiltration	1	
Area	Pop.	Cumu	lative	Peak.	Q _{res}	Area	Accu.	Area	Accu.	Area	Accu.	Q _{C+I+I}	Total	Accu.	Infiltration	Total
		Area	Pop.	Fact.			Area		Area		Area		Area	Area	Flow	Flow
(ha)		(ha)		(-)	(L/s)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(L/s)	(ha)	(ha)	(L/s)	(L/s)
159.5	15360.0	159.5	15360.0	2.77	172.22	14.0	14.0	12.6	12.6	0.00	0.00	23.1	186.1	186.1	52.1	247.
_	Area (ha)	Area Pop.	Area Pop. Cumu Area (ha) (ha)	Area Pop. Cumulative Area Pop. (ha) (ha)	Area Pop. Fact. (ha) (ha) (-)	Area Pop. Cumulative Peak. Q _{res} Area Pop. Fact. (ha) (ha) (-) (L/s)	Area Pop. Cumulative Peak. Q _{res} Area Area Pop. Fact. (ha) (-) (L/s) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Pop. Fact. Area (ha) (-) (L/s) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Area Pop. Fact. Area Area (ha) (-) (L/s) (ha) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Pop. Fact. Area Area Area (ha) (ha) (-) (L/s) (ha) (ha) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area (ha) (ha) (-) (L/s) (ha) (ha) (ha) (ha) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Area Area (ha) (ha) (-) (L/s) (ha) (ha) (ha) (ha) (ha) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Accu. Area Area (ha) (ha) (-) (L/s) (ha) (ha) (ha) (ha) (ha) (ha) (ha) (ha) (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Accu. Area (ha) (ha) (-) (L/s) (ha) (ha) (ha) (ha) (ha) (ha) (L/s) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Accu. Q _{C+H-I} Total Accu. Area Pop. Fact. Area Area Area Area Area Area (ha) (ha)	Area Pop. Cumulative Peak. Q _{res} Area Accu. Area Accu. Area Accu. Area Flow (ha) (L/s)

^{*} See Figure-5 assumes 3.2 people per unit



LEGEND

STUDY AREA

URBAN GROWTH BOUNDARY





Cardinal Creek VILLAGE CATAMARACK TAMARACKHOMES.COM

Richard W. Harrison & Associates Consultancy in Urban Planning & Land Development





SMART SUBDIVISIONS™





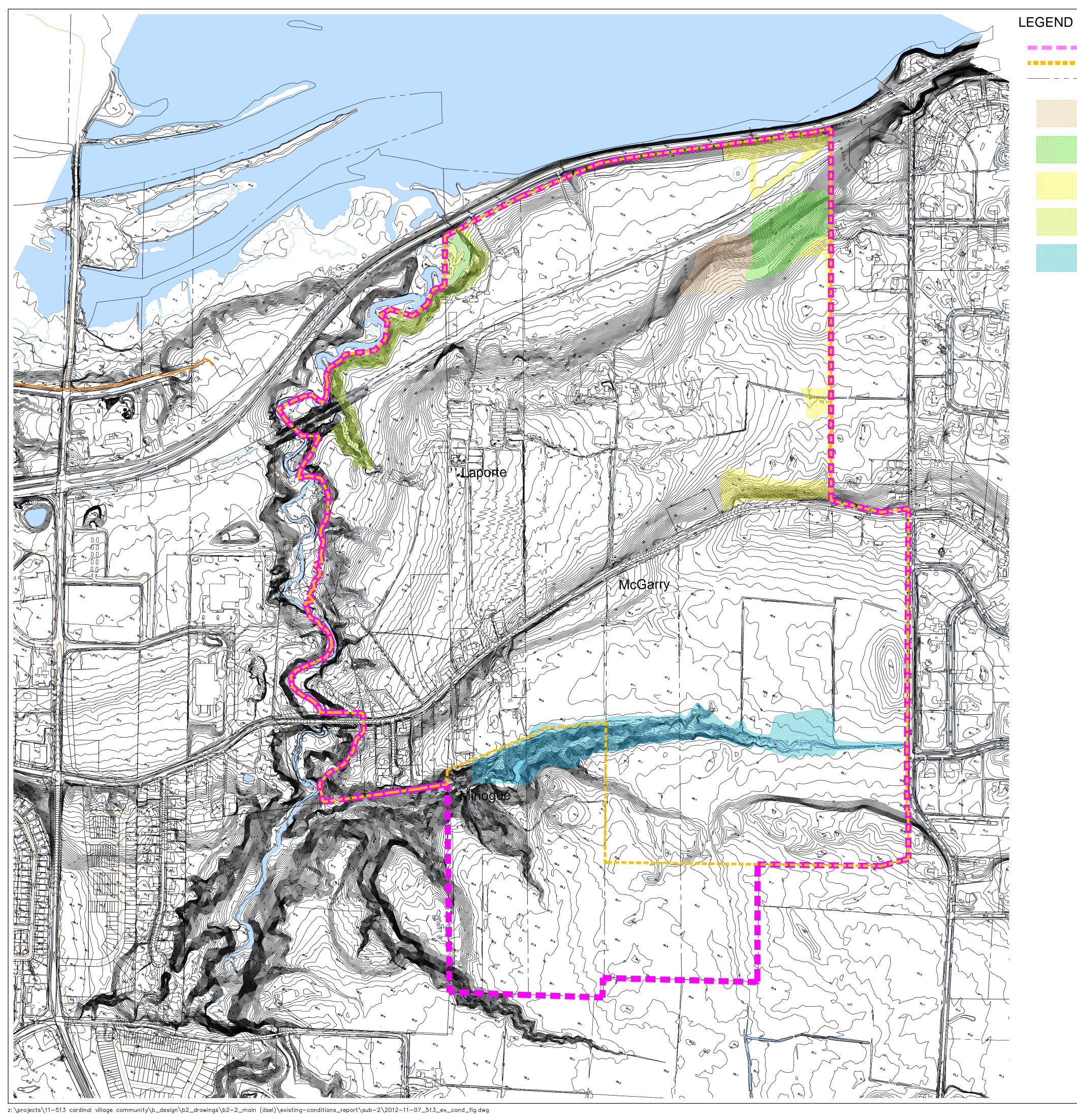


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LM 12.11.07 Existing Conditions Report - November 2012 No. BY YY.MM.DD DESCRIPTION

STUDY AREA

A.D.F. CHECKED BY: S.J.P. DRAWING NO. 1 of 6 1: 5000



STUDY AREA

URBAN GROWTH BOUNDARY PROPERTY LINES

CEDAR FOREST

MAPLE FOREST



OTHER COMPONENTS OF NATURAL SYSTEM



CARDINAL CREEK FEATURE



SOUTH TRIBUTARY CORRIDOR



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2 of 6

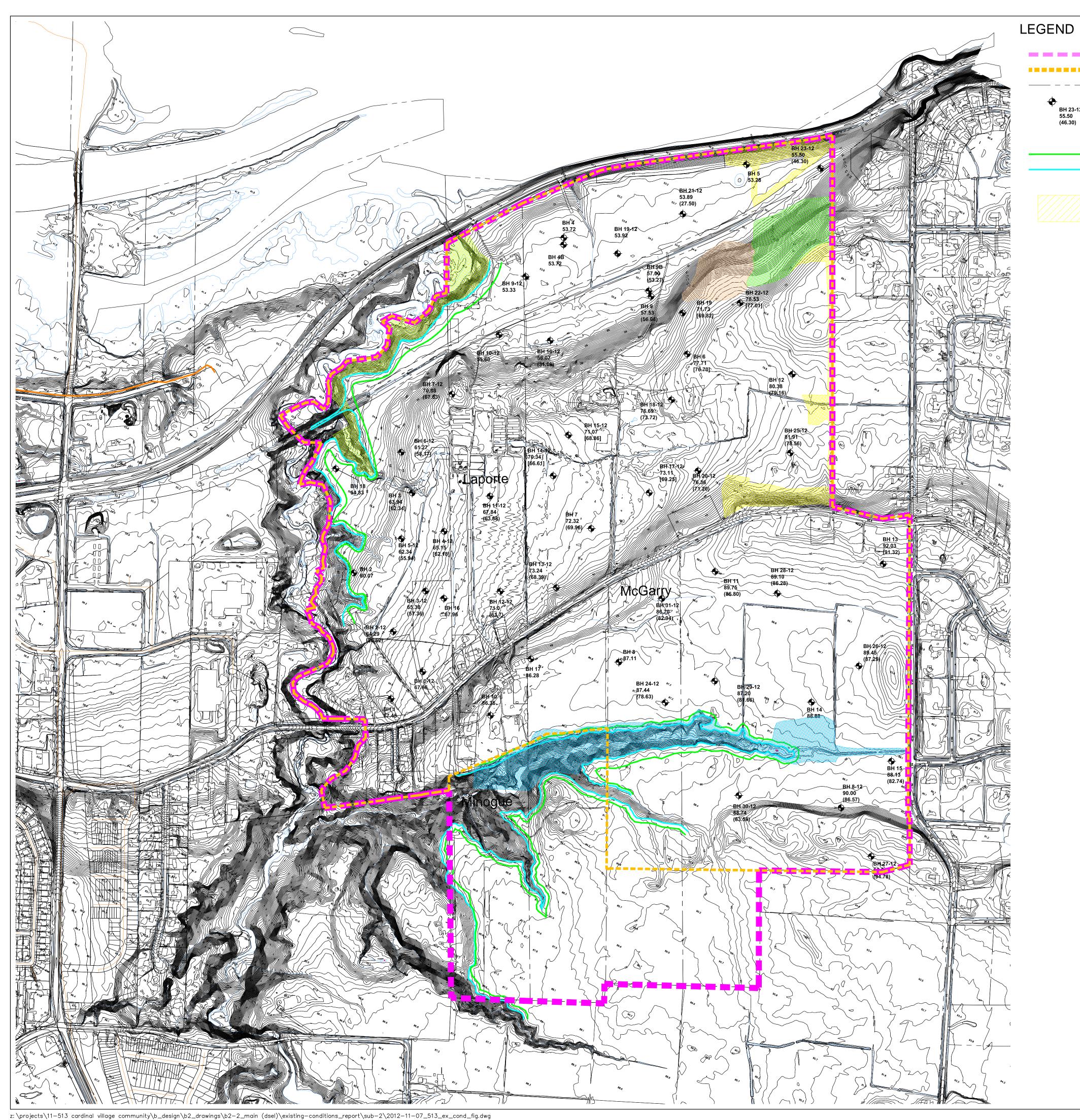
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No. BY YY.MM.DD DESCRIPTION

ENVIRONMENTAL FEATURES PLAN

A.D.F. CHECKED BY: S.J.P. DRAWING NO. FIGURE-2



STUDY AREA

URBAN GROWTH BOUNDARY

PROPERTY LINES

BOREHOLE LOCATIONS (PATERSON GROUP — PG1796) GROUND ELEVATION (PRACTICAL REFUSAL TO AUGERING) [INFERRED BEDROCK ELEVATION]

LIMIT OF HAZARD (PATERSON GROUP - PG1798) TOP OF SLOPE (PATERSON GROUP - PG1798)



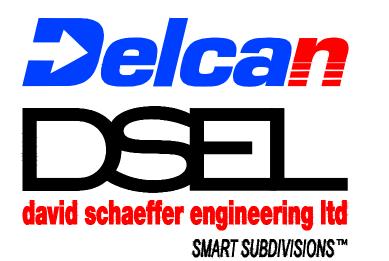
TERRESTRIAL AREA (MUNCASTER ENVIRONMENTAL)



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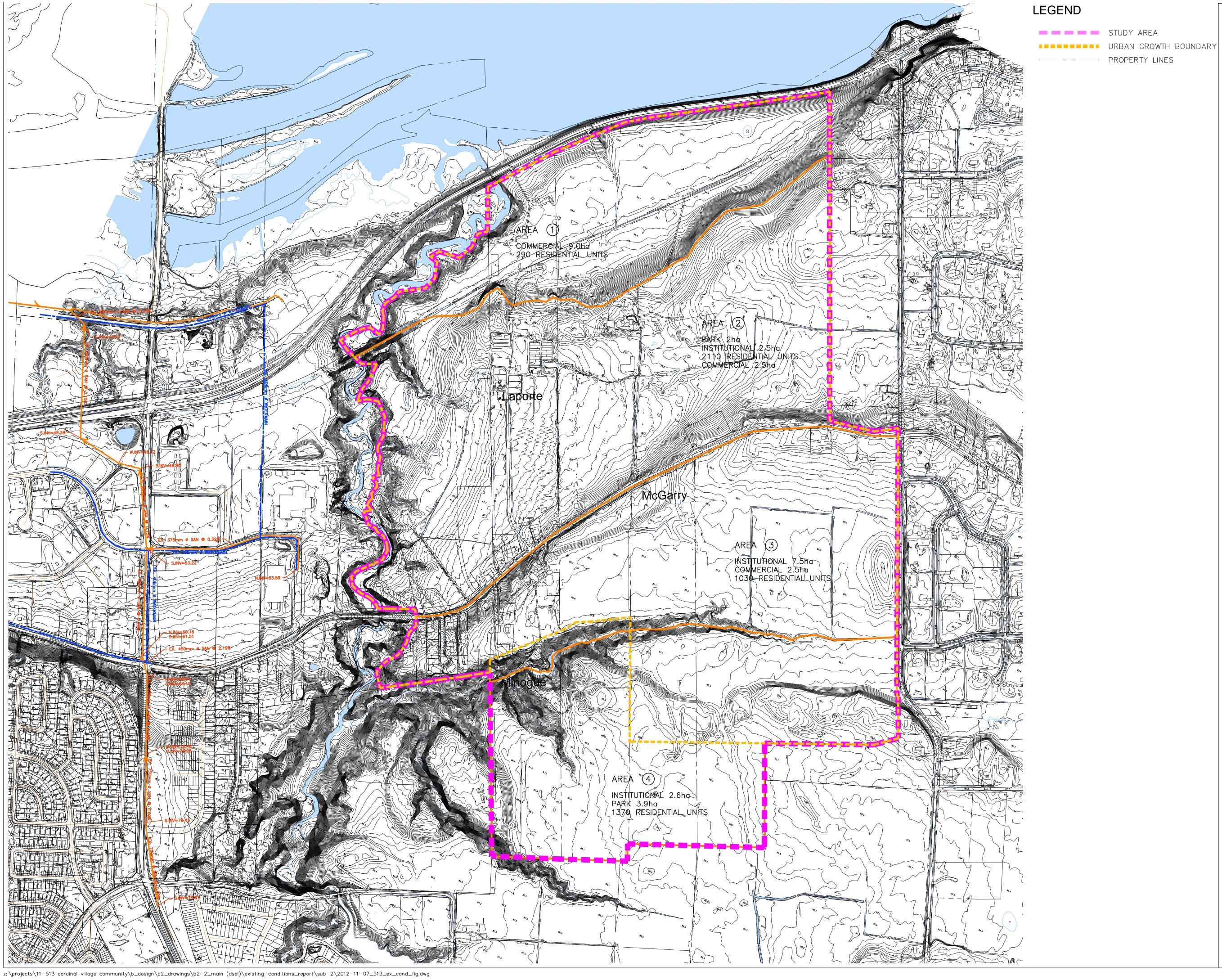
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GEOTECHNICAL PLAN

FIGURE-3 3 of 6 1:5000





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PROPOSED DEVELOPMENT

LEGEND

STUDY AREA

URBAN GROWTH BOUNDARY PROPERTY LINES

TRUNK SEWER



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			SANITA	AKY A	ANALYSIS		

